

Pipinich/Downs

Underground Propane Tank Ballast Weight, Anchor Pullout Strength

Worst case buoyancy for this 500 gallon propane tank which weighs 949 pounds (empty of product) is that it could develop an **uplifting force of 3224 pounds** when fully submerged in ground water.

This upward lift would be restrained by a concrete ballast weight and a group of 4 anchors. The anchors are forged galvanized eye-bolts ½” in diameter and 10” long. The threaded ends are embedded 9” deep [H] into reinforced concrete and are terminated in square galvanized foundation plate washers 3” x 3” and 3/16” thick. The plates are secured to the eye bolts, top and bottom with galvanized round washers and nuts.

The eyebolts are forged closed and are rated at 2200 pounds each.

The anchors are embedded 9” into 14.75” thick reinforced concrete. The anchors are spaced such that there is no loss of efficiency due to spacing. Efficiency loss due to edge spacing 0.638.

Pull out Strength Anchor Calculation: 183,672.26 pounds

Forged eyebolts will fail before concrete.

$$\text{Force lbs} = 800 \text{ psi} \times \pi \times 1.4142 \times H^2 \times 0.638$$

GIVEN:

Base Material Type: Normal Weight Concrete

Base Material Mechanical Properties: $f'_c = 4000$ psi or higher

Base Material Temperature: $T = \text{NA}$

Minimum Base Material Thickness = 14.75 inches

Applied Tension Load per Anchor: $T_{\text{applied}} = 3224$ lb.

Applied Shear Load per Anchor: $V_{\text{applied}} = 3224$ lb.

Anchor Spacing:

$S_1 = 37.4$ inches

$S_2 = \text{NA}$

$S_3 = 60$ inches

$S_4 = \text{NA}$

$S_5 = 70.70$ inches

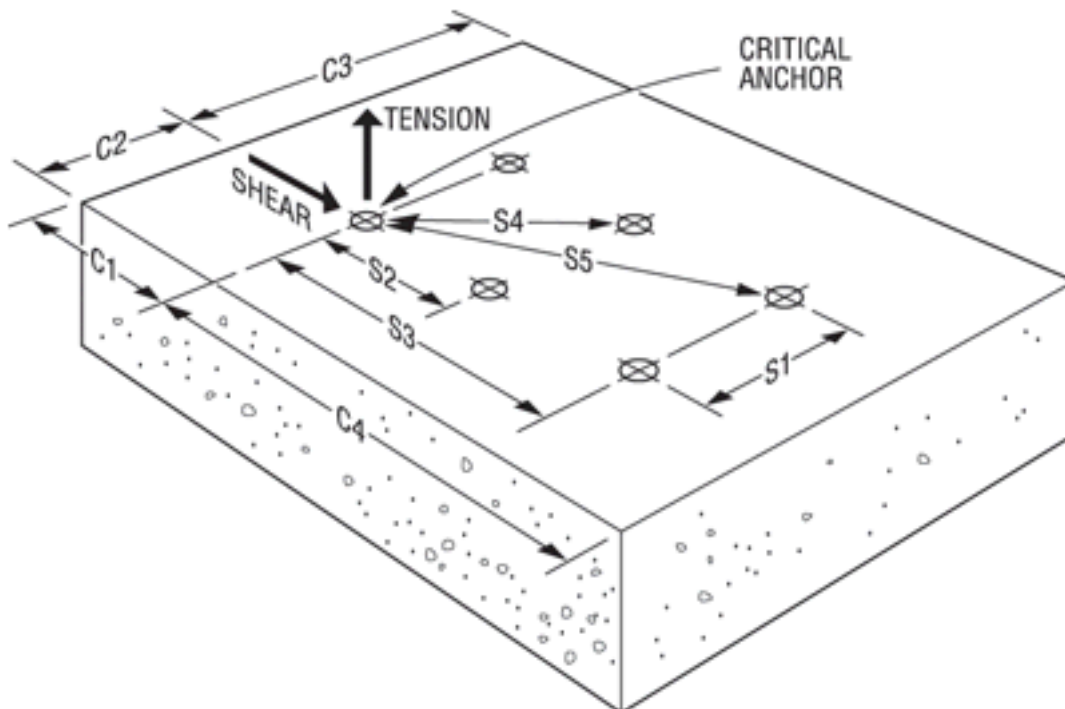
Anchor Edge Distance:

$C_1 = 33$ inches

$C_2 = 6.9$ inches

$C_3 = 44.3$ inches

$C_4 = 93$ inches



CALCULATIONS:

OPTION 1

Proposed Anchor System = Stainless-Steel Wedge-All

Anchor Diameter (in.) or Rebar Size = 1

Embedment Depth = 9 inches

Temperature Sensitivity Factor: $f_{temp} = 1.00$

Short-Term Load Adjustment Factor for Seismic/Wind**: $f_{seismic/wind} = \text{Tension: } 1.00$

Shear: 1.00

**Where permitted by code

Allowable Tension:

$$T = 6770 \text{ lb.}$$

Reduced Efficiency Based on Spacing:

$$f_{S1} = 0.70 + [(1 - 0.70) \times (37.4 - 4.5) / (12.625 - 4.5)] \leq 1, f_{S1} = 1.000$$

$$f_{S2} = 1 \text{ as } S2 = \text{NA}$$

$$f_{S3} = 0.70 + [(1 - 0.70) \times (60 - 4.5) / (12.625 - 4.5)] \leq 1, f_{S3} = 1.000$$

$$f_{S4} = 1 \text{ as } S4 = \text{NA}$$

$$f_{S5} = 0.70 + [(1 - 0.70) \times (70.70 - 4.5) / (12.625 - 4.5)] \leq 1, f_{S5} = 1.000$$

$$(f_S)_{\text{tot.}} = 1.000 \times 1.000 \times 1.000 \times 1.000 \times 1.000 = 1.000$$

Reduced Efficiency Based on Edge/End Distance:

$$f_{C1} = 0.70 + [(1 - 0.70) \times (33 - 4) / (10 - 4)] \leq 1, f_{C1} = 1.000$$

$$f_{C2} = 0.70 + [(1 - 0.70) \times (6.9 - 4) / (10 - 4)] \leq 1, f_{C2} = 0.845$$

$$f_{C3} = 0.70 + [(1 - 0.70) \times (44.3 - 4) / (10 - 4)] \leq 1, f_{C3} = 1.000$$

$$f_{C4} = 0.70 + [(1 - 0.70) \times (93 - 4) / (10 - 4)] \leq 1, f_{C4} = 1.000$$

$$(f_C)_{\text{tot.}} = 1.000 \times 0.845 \times 1.000 \times 1.000 = 0.845$$

T_{design} is equal to:

$$T_{\text{design}} = 6770 \times 1.000 \times 0.845 \times 1.00 \times 1.00 = 5721 \text{ lb.} \geq T_{\text{applied}}, \text{ OK}$$

Allowable Shear:

$$V = 7295 \text{ lb.}$$

Reduced Efficiency Based on Spacing:

$$\begin{aligned}
fS1 &= 0.79 + [(1 - 0.79) \times (37.4 - 4.5) / (12.625 - 4.5)] \leq 1, fS1 = 1.000 \\
fS2 &= 1 \text{ as } S2 = \text{NA} \\
fS3 &= 0.79 + [(1 - 0.79) \times (60 - 4.5) / (12.625 - 4.5)] \leq 1, fS3 = 1.000 \\
fS4 &= 1 \text{ as } S4 = \text{NA} \\
fS5 &= 0.79 + [(1 - 0.79) \times (70.70 - 4.5) / (12.625 - 4.5)] \leq 1, fS5 = 1.000 \\
(fS)_{\text{tot.}} &= 1.000 \times 1.000 \times 1.000 \times 1.000 \times 1.000 = 1.000
\end{aligned}$$

Reduced Efficiency Based on Edge/End Distance:

$$\begin{aligned}
fC1 &= 0.30 + [(1 - 0.30) \times (33 - 4) / (10 - 4)] \leq 1, fC1 = 1.000 \\
fC2 &= 0.30 + [(1 - 0.30) \times (6.9 - 4) / (10 - 4)] \leq 1, fC2 = 0.6383 \\
fC3 &= 0.30 + [(1 - 0.30) \times (44.3 - 4) / (10 - 4)] \leq 1, fC3 = 1.000 \\
fC4 &= 0.30 + [(1 - 0.30) \times (93 - 4) / (10 - 4)] \leq 1, fC4 = 1.000 \\
(fC)_{\text{tot.}} &= 1.000 \times 0.6383 \times 1.000 \times 1.000 = 0.6383
\end{aligned}$$

Allowable Shear Value for Design:

V_{design} is equal to:

$$V_{\text{design}} = 7295 \times 1.000 \times 0.6383 \times 1.00 \times 1.00 = 4657 \text{ lb.} \geq V_{\text{applied}}, \text{ OK}$$

Check Combined Tension & Shear Interaction:

$$(T_{\text{applied}}/T_{\text{design}})^n + (V_{\text{applied}}/V_{\text{design}})^n \leq \text{or} = 1.0$$

$$(3224/5721)^{1.666667} + (3224/4657)^{1.666667} = 0.926357 \leq \text{or} = 1.0 \text{ OK}$$

Where $n = 5/3$ for Carbon/Stainless Steel Wedge-All, Titen HD and Carbon/Stainless Steel Drop-In anchors installed in Normal Weight Concrete only. Otherwise, $n = 1$ for all other cases.